Stormwater Report

Milwaukie Riverfront Park

City of Milwaukie

Prepared For: City of Milwaukie

Prepared By: David Evans and Associates, Inc.

December 2009

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City of Milwaukie

Prepared for:

City of Milwaukie JoAnn Herrigel, Community Services Director 10722 SE Main Street Milwaukie, OR 97222

Prepared by:

David Evans and Associates, Inc. 2100 SW River Parkway Portland, Oregon 97201

DEA Project Number: MAEX0000-0019

December 2009

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1 INTRODUCTION

The City of Milwaukie Riverfront Park Project is located between Mcloughlin Blvd. and the Willamette River at the Kellogg Creek inflow. The City has long desired to improve pedestrian access to Riverfront recreational facilities and to reconnect the downtown business and retail areas to Riverfront. Improvements to the park include an amphitheater, restrooms, plaza, boat parking, car parking, pedestrian suspension bridge, boat ramp and dock, floating dock, pavilion with overlook, and multi-use paths.

The project is located in the City of Milwaukie and falls under the administration of the City of Milwaukie (COM) Public Works Design Standards for stormwater design. This report addresses the stormwater design of the park improvements in relation to COM stormwater regulations.

2 EXISTING SITE

The existing park has two parking areas and a boat ramp. The majority of the existing impervious areas will be removed for the new layout of parking and sidewalks. The vehicular bridge crossing Kellogg Creek and connecting the north and south parking areas will remain and will be incorporated into the site improvements. The total predevelopment site impervious area is approximately 103,960 square-feet (2.4 acres). An impervious area of 95,756 square-feet (2.2 acres) will be removed. An impervious area of 8,204 square-feet (0.2 acres) of sidewalk will remain. For the most part there is not an existing storm system. Stormwater from the impervious surfaces flows down the river bank to the Willamette River. However, there are two existing catch basins that collect part of the driveway stormwater runoff. These catch basins will be removed during construction since the driveway will be relocated.

3 PROPOSED IMPROVEMENTS

The Riverfront Park project has multiple uses including large grassy areas, picnic facilities, plaza with restrooms, amphitheater, benches for viewing the river, natural vegetative areas with trails, a boat ramp and parking, and transient boat dock. The site has 20 long parking spaces for vehicles with trailers and 16 standard size parking spaces. The site improvements will add 122,821 square-feet (2.8 acres) of impervious area to the existing 8,204 square-feet (0.2 acres) that will remain. The total impervious area for the site post-development will be 131,025 square-feet (3.0 acres). The project will create a net increase of 27,065 square-feet (0.6 acres) of impervious area. For discussion of stormwater the site has been divided into three key areas. These areas are the north and south parking (intersected by Kellogg creek connected by an existing vehicle bridge and

the proposed pedestrian bridge) and the north pedestrian plaza. All storm water from vehicular impervious surfaces on the site will be collected and all storm water up to the 10-year event will be treated and infiltrated on the site. Overflow from larger storms, and runoff from some non-vehicular surfaces will be discharged at six pipe outfalls into the Willamette River.

Appendix A shows the existing site plan and the proposed site layout and storm design.

4 HYDROLOGIC PARAMETERS

Table 4.1 details the 24-hour rainfall amounts for the City of Milwaukie taken from Oregon's isopluvial maps published by NOAA.

Recurrence Interval (years)	Total Rainfall (inches)	
2	2.7	
5	3.1	
10	3.4	
25	3.9	
100	4.6	
WQ	0.83	

Table 4.1: 24-Hour Rainfall Amounts for the City of Milwaukie

The existing soil in the project area is Urban Land (Hydrologic Soil Group D). The pervious area curve number (CN) used is 84 (fair condition open space) and the impervious area CN used is 98. An estimated time of concentration of 15.0 minutes was used for existing conditions analysis. The post construction time of concentration was calculated as a 5 minute travel time to water quality and detention facilities plus the travel time through the water quality and detention facilities.

5 PROPOSED WATER QUALITY

COM stormwater regulations state that water quality facilities are required to meet the design standards of the current City of Portland, Stormwater Management Manuel (SWMM). SWMM specifies that pollution reduction is required for all impervious areas created by development projects with the exception of roof areas. SWMM regulations require water quality facilities to treat stormwater runoff generated by 0.83 inches of rainfall over a 24-hour period when using the SBUH hydrograph-based analysis method.

5.1 SOUTH PARKING

Stormwater runoff in the south parking area (Basins C1, C2, C3 and C4) is treated and detained in four (4) separate facilities; one (1) vegetated swale/planter and three (3) infiltration planters. All facilities are designed to meet the water quality treatment requirements specified by the SWMM. The City of Portland, Bureau of Environmental Services (BES), Presumptive Approach Calculator was used to model water quality storm event capacity of all water quality facilities. For Basin Map see Appendix B.

The south parking area includes a plaza overlooking the Willamette River. This overlook is Basin C1 (3,796 square feet impervious). The storm water runoff from Basin C1 is collected by sheet flow into two (2) infiltration planters, Planter #1 and Planter #2. The two planters combine to provide 477 cubic feet (cf) of storage volume (344 cf above grade, and 103 cf in the voids of drain rock below grade). An area drain overflow will be placed 12-inches above the bottom of the planter. The 12 inches of dead storage provides water quality treatment and allows time for the storm water to infiltrate. The planters treat the water quality storm event flow of 0.016 cubic feet per second (cfs) and also fully infiltrate the 10-year storm water runoff of 0.077 cfs. The soils for the project site are classified as Urban Soils by the USDA Soil Conservation Service. Urban Soils are not provided with typical infiltration rates. Soils adjacent to our site that are classified have reported infiltration rates between 0.6 - 2.0 inches per hour. We have assumed an infiltration rate of 2.0 inches per hour.

Runoff from Basin C2 (16,320 square-feet impervious and 3,465 square feet pervious) is conveyed to a vegetated swale, "South Swale," through concrete curb cuts. The swale is located along vehicle turn-around next to the river overlook. The swale is 188 feet long. It has a 0.5-foot bottom width and 3L:1V side slopes. Initially the swale is 1.5 feet deep, but increases to a depth of 3 feet at the outlet point. The South Swale coveys the storm water to Planter #3.

Planter #3 is along the same alignment as South Swale. The remainder of the C2 Basin surface area that does not discharge storm water to the swale is collected into Planter #3 through concrete curb cuts Basin C2 generates a water quality runoff flow rate of 0.067 cfs. Planter #3 has 1,814 cf of storage volume (1,395 cf above grade, and 418 cf below grade). An area drain overflow is placed at an elevation of 12-inches above the bottom of the pond. The 12-inches of dead storage provide water quality and infiltration. The swale and Planter #3 combine to treat a water quality runoff flow rate of 0.067 cfs and also fully infiltrate the runoff from a 10-year event of 0.208 cfs.

Runoff from Basin C3 is conveyed to "Large Pond". The pond is located at the center island between trailer parking and standard parking. Runoff from Basin C3 (10,212 square feet impervious and 3,222 square-feet pervious) is conveyed to Large Pond

through concrete curb cuts. The water quality storm event for Basin C3 generates a runoff of 0.042 cfs. Large Pond has a storage volume of 1,032 cf. The proposed outlet is a ditch inlet with inlet elevation 18 inches above the bottom of the pond. The 18-inch difference creates a dead storage for water quality and infiltration (2.0 inches per hour infiltration was used for modeling). The Large Pond also infiltrates the entire storm water runoff from a 10-year storm event, 0.208 cfs

Basin C4 includes the roadway area south of the bridge crossing Kellogg Creek. Runoff from Basin C4 (3,328 sf impervious) is conveyed to Planter #4. Similar to the other planters, there is an area drain placed 12 inches above the bottom of the planter to maximize treatment and infiltration. Planter #4 provides 553 cf of storage volume (425 cf above grade, and 128 cf below grade). The planter provides treatment for the water quality flow of 0.014 cfs and fully infiltrates the runoff from the 10-year event, 0.068 cfs.

For water quality and detention calculations see Appendix C.

5.2 NORTH PARKING

Stormwater runoff in the north parking area (Basins N1, N2, and N3) is treated and detained in three (3) separate facilities; one (1) flat planter and two (2) sloped planters. All three planters are designed to SWMM's water quality requirements.

Basin N1 includes the lower level parking drive aisle and sidewalks. Storm water is conveyed into Planter #5 via trench drain located at the top of the boat ramp and by curb cuts along the drive aisle. Storm water runoff from Basins N1 (11,375 square-feet impervious) generates a water quality runoff flow of 0.047 cfs. Planter #5 is 7 feet wide by 151 feet long and is sloped at 1.5% to match the slope of the adjacent drive aisle. The planter has six (6) check dams spaced equally along the bottom of the planter to maximize the infiltration area. With 12-inches of dead storage and 12-inches of drain rock media below grade, there is approximately 1,205 cf of storage volume (907 cf above grade, and 298 cf below grade). The planter provides treatment for the water quality storm event (0.047 cfs) and fully infiltrates the 10-year storm event runoff (0.232 cfs) with no overflow. An overflow catch basin is providing 12 inches above the bottom of the planter for larger storm events.

Basin N2 (6,372 sf impervious) includes the paved drive aisle directly north of the Kellogg Creek Bridge. Storm water is conveyed to Planter #6 via trench drain. Planter #6 is on the opposite side of the lower drive aisle from Planter #5. And, it is just west of Planter #7. The planter is shaped like a sawtooth and is sloped, similar to Planter #5, at a 1.5-percent grade. Planter #6 also includes check dams to maximize infiltration. The

planter has an average bottom width of 7 feet. The planter has a vertical retaining wall on the east side and has 3L:1V side slopes on the west side. Planter #6 provides treatment for the water quality storm event (0.026 cfs) and fully infiltrates the 10-year storm event runoff (0.130 cfs) with no overflow. An overflow catch basin is provided 12 inches from the bottom of the planter to manage larger storm events.

Basin N3 (3,429 sf impervious, and 6,487 pervious) includes the upper level parking and drive aisle. The parking stall pavement surface is a pervious pavement material to allow for immediate surface infiltration. Storm water runoff from Basin N3 sheet flows across the parking stalls and through curb cuts to Planter #7. The planter zigzags along the front of the parking stall and is 3 feet wide. Planter #7 has 3 inches of dead storage and 12 inches of drain rock media below grade, providing 288 cf of storage volume (131 cf above grade, and 157 cf below grade). Planter #7 provides treatment for the water quality storm event (0.014 cfs) and fully infiltrates the 10-year storm event runoff (0.070 cfs) with no overflow. Overflow notches in the planter wall located 3 inches above the bottom of the planter allow for sufficient overflow for the larger storm events.

For water quality and detention calculations see Appendix C.

5.3 PEDESTRIAN PLAZA

The Pedestrian Plaza area includes the restrooms, water features, planters, and amphitheater these areas have two (2) water quality features for stormwater treatment. A swale is proposed on the south side of the plaza and a filter strip is proposed on the far north side of the plaza. All the facilities meet SWMM's requirements for water quality. A large percentage of the plaza is graded to sheet flow stormwater runoff run into adjacent planters or grassy areas. These areas were not modeled for water quality purposes.

On the south side of the plaza there is a 100-foot long water quality swale with 12-inch high check dams located every 25 feet to allow for higher infiltration. The swale is sloped at approximately 4 percent and the bottom width is 9 feet. The side slopes are 2L:1V. The swale treats a water quality runoff event of 0.044 cfs from Basin P1 (10,660 square-feet of impervious). Basin P1 storm water is collected in area drains and is conveyed to the swale using a 12-inch pipe. The swale also infiltrates 99 percent of the 10-year storm event with a runoff flow rate of 0.07 cfs.

On the south side of the plaza there is an 80-foot long water quality swale with 12-inch high check dams located every 20 feet to allow for higher infiltration. The swale is sloped at approximately 3.4 percent and the bottom width is 4 feet. The side slopes are 2L:1V. The swale treats a water quality runoff event of 0.009 cfs from Basin P2

(2,145 square-feet of impervious). Basin P2 storm water is collected by area drains and is conveyed to the swale using a 12-inch pipe. The swale also infiltrates 100 percent of the 10-year storm event with a runoff flow rate of 0.044cfs.

Both swales treating storm water from the plaza areas eventually discharge to the Willamette River.

For water quality and detention calculations see Appendix C.

6 PROPOSED DETENTION

Since infiltration rates in this area are not high enough to infiltrate the 2-, 5-, 10-, and 25-year storm event, it is necessary to provide a detention system in order to meet the COM flow attenuation requirements: post-development flow for 2-, 5-, 10-, and 25-year storm events shall be detained to the pre-development discharge rate. The detention facilities for the project have been designed to meet the COM requirements.

Table 6.0 is a summary of the pre and post development runoff flow-rates for the 2-, 5-, 10-, and 25-year storm events.

Recurrence Interval (years)	Pre-Development Flow (cfs)	Post- Development Flow (cfs)	Flow Reduction (cfs)
2	1.31	0.96	0.35
5	1.52	1.17	0.35
10	1.67	1.38	0.29
25	1.93	1.59	0.34

Table 6.0: Pre- and Post-Development Flow and Detention Requirements

6.1 SOUTH PARKING

Table 6.1.1 is a summary of the proposed South Parking area water quality and detention facilities. Inflow is the sum of flow from all basins (Basins C1, C2, C3 and C4). Modeled outflow is the post-development flow released from the four (4) separate facilities; one (1) vegetated swale/planter and three (3) infiltration planters.

Recurrence Interval (years)	Inflow (cfs)	Modeled Outflow (cfs)	Flow Reduction (cfs)
2	0.52	0.48	0.02
5	0.61	0.58	0.03
10	0.68	0.68	0.00
25	0.79	0.79	0.00

Table 6.1.1: South Parking Detention Summary

The total storm water detention storage volume for the South Parking treatment facilities is approximately 3,846 cubic feet.

For water quality and detention calculations see Appendix C.

6.2 NORTH PARKING

Table 6.2.1 is a summary of the proposed North Parking area water quality and detention facilities. Inflow is the sum of flow from all the basins (Basin N1, N2, and N3). Modeled outflow summation of the flow that is released from the three (3) facilities; one (1) flat planter and two (2) sloped planters.

Recurrence Interval (years)	Inflow (cfs)	Modeled Outflow (cfs)	Flow Reduction (cfs)
2	0.42	0.30	0.12
5	0.49	0.37	0.12
10	0.55	0.43	0.12
25	0.62	0.50	0.12

 Table 6.2.1: North Parking Detention Summary

The total storm water detention storage volume for the North Parking treatment facilities is approximately 1,822 cubic feet.

For water quality and detention calculations see Appendix C.

6.3 PEDESTRIAN PLAZA

The proposed Pedestrian Plaza area water quality swales provide treatment and some detention value; however, we did not include detention in our analysis. The detention value is in the form of check dams spaced 20 to 25 feet apart. The total storm water detention storage volume for the Pedestrian Plaza treatment facilities is approximately 814 cubic feet.

For water quality and detention calculations see Appendix C.

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Appendix A

Existing Site Plan and Proposed Site Plan







Appendix B

Basin Map



Appendix C

Water Quality and Detention Analysis

Project Name:

Project Address:

Presumptive Approach Calculator ver. 1.1

Catchment Data

Catchment ID:

C1 Date: 03/15/09 Permit Number:

Designer: Company:

Milwaukie Park - Overlook Catch Milwaukie, OR SDH David Evans and Associates, Inc.

Run Time: 5/13/2009 10:56:06 AM

Drainage Catchment Information	
Catchment ID	GI
	Catchment Area
Impervious Area	3,796 SF
Impervious Area	0.09 ac
Impervious Area Curve Number, CN _{imp}	98
Time of Concentration, Tc, minutes	5 min.
Site Soils & Infiltration Testing Data	
Infiltration Testing Procedure: Open Pil	Falling Head
Native Soil Field Tested Infiltration Rate (Itest):	2 in/hr
Bottom of Facility Meets Required Separation From	
High Groundwater Per BES SWMM Section 1.4:	Yes
Correction Factor Component	
CF _{test} (ranges from 1 to 3)	2
Design Infiltration Rates	
I _{dsgn} for Native (I _{test} / CF _{test}):	1.00 in/hr
I _{dsgn} for Imported Growing Medium:	2.00 in/hr

Execute SBUH Calculations



Facility Design Data

Calculation Guide

Max. Rock Stor.

Bottom Area

344 SF



Goal Summary:

FACILITY FACTS

Total Facility Area Including Freeboard =

Sizing Ratio (Total Facility Area / Catchment Area) =

344 SF

0.091



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Presumptive Approach Calculator ver. 1.1

Catchment Data

Catchment ID:

C2 Date: 03/15/09 Permit Number:

Project Name: Project Address:

Designer:

Company:

Milwaukie Park - So. Parking1 Catch Milwaukie, OR SDH David Evans and Associates, Inc.

Run Time: 5/13/2009 11:09:52 AM

Drainage Catchment Information Catchment ID C2 **Catchment Area** Impervious Area 16,320 SF Impervious Area 0.37 ac Impervious Area Curve Number, CNimo 98 Time of Concentration, Tc, minutes 5 min Site Soils & Infiltration Testing Data 1.00 Service. Infiltration Testing Procedure: Open Pit Falling Head Native Soil Field Tested Infiltration Rate (Itest): 2 in/hr Bottom of Facility Meets Required Separation From High Groundwater Per BES SWMM Section 1.4: Yes **Correction Factor Component** CF_{test} (ranges from 1 to 3) 2 **Design Infiltration Rates** Idsgn for Native (Itest / CFtest): 1.00 in/hr Idsgn for Imported Growing Medium: 2.00 in/hr

> Execute SBUH Calculations





PR Con-A&B

BES - Presumptive Approach Calculator - Ver 1.1



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Presumptive Approach Calculator ver. 1.1

Catchment Data

Catchment ID:

C3 Date: 03/15/09 Permit Number: -

Project Name: Project Address:

Designer: Company: Milwaukie Park - No. Parking2 Catch Milwaukie, OR SDH David Evans and Associates, Inc.

Run Time: 5/13/2009 12:54:02 PM

Drainage Catchment Information Catchment ID C3 Catchment Area Impervious Area 10,212 SF 0.23 Impervious Area lac Impervious Area Curve Number, CNimp 98 Time of Concentration, Tc, minutes 5 min. Site Soils & Infiltration Testing Data Infiltration Testing Procedure: Open Pit Falling Head Native Soil Field Tested Infiltration Rate (Itest): 2 in/hr Bottom of Facility Meets Required Separation From High Groundwater Per BES SWMM Section 1.4: Yes Correction Factor Component CF_{test} (ranges from 1 to 3) 2 **Design Infiltration** Rates Idsgn for Native (Itest / CFtest): 1.00 in/hr Idson for Imported Growing Medium: 2.00 in/hr

> **Execute SBUH** Calculations







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10-yr Con-A&B



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Presumptive Approach Calculator ver. 1.1

Catchment Data

C4

Catchment ID:

Project Name: Project Address:

Designer:

Company:

Milwaukie Park - Access Catch -Milwaukie, OR SDH David Evans and Associates, Inc. Date: 03/15/09 Permit Number: 0

Run Time: 5/13/2009 12:35:49 PM

Drainage Catchment Information		
Catchment ID	C4	1 1
	atchment A	a rea
Impervious Area	3,328	
Impervious Area	0.08	ac
Impervious Area Curve Number, CN _{imp}	98	
Time of Concentration, Tc, minutes	5	min.
Site Soils & Infiltration Testing Data	as Dependence	
Infiltration Testing Procedure: Open Pit	Falling Head	
Native Soil Field Tested Infiltration Rate (Itest):	<u> 2</u>	in/hr
Bottom of Facility Meets Required Separation From		
High Groundwater Per BES SWMM Section 1.4:	Yes	
Correction Factor Component		
CF _{test} (ranges from 1 to 3)	2	
Design Infiltration Rates		
I _{dsgn} for Native (I _{test} / CF _{test}):	1.00	in/hr
l _{dsgn} for Imported Growing Medium:	2.00	in/hr

Execute SBUH Calculations



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PR Con-A&B



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10-yr Con-A&B



Company:

Presumptive Approach Calculator ver. 1.1

Catchment Data

Project Name:	Milwaukie Park - Lower No. Parking Ca
Project Address:	
	Milwaukie, OR
Designer:	SDH

David Evans and Associates, Inc.

Catchment ID:	N1
Date:	03/15/09
Permit Number:	0

Run Time: 5/14/2009 11:42:05 AM

Execute SBUH

Drainage Catchment Information		
Catchment ID	N1	
	Catchment A	rea
Impervious Area	11,375	SF
Impervious Area	0.26	ac
Impervious Area Curve Number, CN _{imp}	98	
Time of Concentration, Tc, minutes	5	min.
Site Solls & Infiltration Testing Data		
Infiltration Testing Procedure: Open Ri	t Falling Head	
Native Soil Field Tested Infiltration Rate (Itest):	2	in/hr
Bottom of Facility Meets Required Separation From		
High Groundwater Per BES SWMM Section 1.4:	Yes	
Correction Factor Component		
CF _{test} (ranges from 1 to 3)	2	
Design Infiltration Rates		
I _{dsgn} for Native (I _{test} / CF _{test}):	1.00	in/hr
Idson for Imported Growing Medium:	2.00	in/hr



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Facility D	esign)	Data
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PR Con-A&B



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Sloped Facility Worksheet

	Presumptive Approach Calculator	ver. 1.1	Catchment Data	
		Catchment ID:	N2	
Project Name:	Milwaukie Park - Bridge to Parking Cate	Date:	03/15/09	
Project Address:	• ·	Permit Number: 0		
	Milwaukie, OR	Run Time: 12/8/2	009 12:20:16 PM	
Designer:	SDH	Run Hime. 32/0/20	JUS 12.20. 10 P W	
Company:	David Evans and Associates, Inc.			

Diamage Catchinent Information		
Catchment ID	N2	
(Catchment Area	
Impervious Area	6,372 SF	
Impervious Area	0.15 ac	
Impervious Area Curve Number, CN _{imp}	98	
Time of Concentration, Tc, minutes	5 min.	
Site Soils & Infiltration Testing Data		
Infiltration Testing Procedure: Open Pit	t Falling Head	
Native Soil Field Tested Infiltration Rate (Itest):	4 in/hr	
Bottom of Facility Meets Required Separation From		
High Groundwater Per BES SWMM Section 1.4:	Yes	
Correction Factor Component		
CF _{test} (ranges from 1 to 3)	2	
Design Infiltration Rates	**************************************	
I _{dsgn} for Native (I _{test} / CF _{test}):	2.00 in/hr	
l _{dsgn} for Imported Growing Medium:	2.00 in/hr	

Execute SBUH



ļ		Presumptive Approach	Calculato	r ver. 1.1		hment ID:	N2	
Proj	ject Name:	: Milwaukie Park - Bridge to Parki	ng Catch C	atchment ID:	<u>N2</u> D	ate:	3/15/2009	
2 2 5 Catchment t	 Select Failed and slop Select ty Select ty Complete facility will 	which Stormwater Hierarchy Categor acility Type. acility shape of surface facility to mo ed planters that use the PAC Sloped pe of facility configuration. e data entry for all highlighted cells.	re accurately e		ne, except for S	wales		
Goal Summ	iary:				I	1		
Hierarchy Category		SWMM Requirement	RESULTS box Pollution Reduction as a	below needs to display 10-yr (aka disposal) as a	Facility configurations allowed			
1	On-site in	filtration with a surface infiltration facility.	PASS	PASS	A or B			
Facil	lity Type =	Planter (Sloped)	Facility C		A			
	Works	o Sloped Facility sheet and enter ble Parameters	Bott			A		Calculation Guide
	Infiltra	GRADE STORAGE COMPONENT ation Area = <u>481</u> sf ty Volume = <u>329.0</u> cf		<u>BELOW GRA</u> Rock Storage Bottor Rock Storage	· · ·	1sf		Max. Rock Stor. Bottom Area Per Swale Dims
Gre	-	ium Depth ≕ <u>18</u> in ard Depth ≃ <u>N/A</u> in						
	esign Infiltr	at Depth 1 = 329 cf ation Rate = 2.00 in/hr h Capacity = 0.022 cfs	٩	Rock Storage Ca Vative Design Infiltration Infiltration Ca	n Rate =2.0	0 in/hr	GM Infiltration	Rate Used in PAC
	RESULTS Pollution Reduction 10-yr FACILITY I		Cap. Used	Br	urrent data ha id Parking2 I		orted: 12/8/2009	
	Si	Total Facility Area Includin zing Ratio (Total Facility Area / Cato	-		10_Farking2_1 2:21:33 PM	LAPOI 6A13	1210(2000	





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Presumptive Approach Calculator ver. 1.1

Catchment Data

N3

Project	Name:
Project	Address:

Milwaukie Park - Upper No. Parking Ca

				2.6.7	1. 1910		9		n:://:
Mily	/aul	cie,	ÔR						6396
SDF									Sec. South
Dav	id E	van	s ai	nd /	Asso	ocia	tes.	Inc.	

Catchment ID:

Date: 03/15/09 Permit Number: 0

Run Time: 5/14/2009 12:37:10 PM

Designer: Company:

Drainage Catchment Information								
Catchment ID	N3							
Catchment Area								
Impervious Area	3,429	SF						
Impervious Area	0.08	ac						
Impervious Area Curve Number, CN _{imp}	98							
Time of Concentration, Tc, minutes	5	min.						
Site Soils & Infiltration Testing Data	an contractor							
Infiltration Testing Procedure: Open Pit	Falling Head							
Native Soil Field Tested Infiltration Rate (Itest):	2	in/hr						
Bottom of Facility Meets Required Separation From								
High Groundwater Per BES SWMM Section 1.4:	Yes							
Correction Factor Component								
CF _{test} (ranges from 1 to 3)	2							
Design Infiltration Rates								
I _{dsgn} for Native (I _{test} / CF _{test}):	1.00	in/hr						
I _{dsgn} for Imported Growing Medium:	2.00	in/hr						

Execute SBUH Calculations



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Pro	ject Name:	Presum Milwaukie	-				or ver. 1.1 Catchment ID	: 1	Run Time:	chment ID 5/14/2009 ⁻ Date:		
Catchment	Instruction 1. Identify w 2. Select Fa 3. Identify fa and slope 4. Select typ 5. Complete facility will r	s: which Stormv acility Type. acility shape	vater Hier of surfac hat use ti configura for all hig	rarchy C ce facility he PAC ation. hlighted	ategory to mor Sloped cells.	the facility.	estimate surface ksheet to enter	ce volum			5752003	
Goal Summ	ary:									J		
Hierarchy Category		SWMM	Requirem	ent	-	RESULTS be Pollution Reduction as	ox below needs to d		Facility configurations allowed			
1 -	On-site infi	iltration with a	surface infi	Itration fac	ility.	PASS	PASS		A or B	-		
		Planter (Fla Rectangle/		<u>.</u>		Facility	Configuration		B	-		
			-Facility B Area	Bottom		- 	PLANTER -		ge Depth 1 SM Depth			Calculation Guide Max. Rock Stor.
	ABOVE G Facility Bott	RADE STOR	Contraction of the second strains	OMPON sf	<u>ENT</u>		BELO Rock Storag		DE STORAGI Area = 52	-		Bottom Area
	Botto Facility Si Storage owing Mediu		3.0 0 3	ft to 1 in	<warr< th=""><th>ning</th><th>Rock</th><th>Storage</th><th>Depth =1 Ratio =0</th><th>2</th><th></th><th>522 SF</th></warr<>	ning	Rock	Storage	Depth =1 Ratio =0	2		522 SF
	Capacity at sign Infiltrat Infiltration	tion Rate =	131 2.00 0.024	cf in/hr cfs			Native Design I	orage Caj nfiltration ration Caj	Rate = 1.0	0 in/hr		
	Pollution Reduction [10-yr [ACILITY FA	PASS PASS	Overflow Volume 0 CF 0 CF	1% 55% 100%	Rock C Surf. C Rock C	ap. Used ap. Used ap. Used ap. Used	Rin PAC					
	Sizi	Tota ing Ratio (To				Freeboard = nent Area) =						

Facility Design Data



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	Presumptive Approach Calcul	Catchment Data	
		Catchment	ID: P1
Project Name:	Milwaukie Park - Plaza 1	Da	te: 12/10/09
Project Address:		per: 0	
-	Milwaukie, OR	Run Time: 1	2/10/2009 3:05:27 PM
Designer:	SDH	, , , , , , , , , , , , , , , , ,	
Company:	David Evans and Associates, Inc.	· · · · · · · · · · · · · · · · · · ·	
Drainage Catchmo	ent Information	ak da katalan da katal Katalan da katalan da ka Katalan da katalan da ka	tana Marina Marina ang ang ang ang ang ang ang ang ang a
Catchment ID	P1		
	Catchment Area		

		Acountion A	u -	
Impervious Area		10,660	SF	
Impervious Area		0.24	ac	
Impervious Area Curve Number, CN _{in}	98			
Time of Concentration, Tc, minutes		5	min.	
Site Soils & Infiltration Testin	g Data			
Infiltration Testing Procedure:	Open Pit	Falling Head		
Native Soil Field Tested Infiltration Ra	te (I _{test}):	4	in/hr	
Bottom of Facility Meets Required Ser				·
High Groundwater Per BES SWMM S		Yes		
Correction Factor Component				
CF _{test} (ranges from 1 to 3)		2		
Design Infiltration Rates				
I _{dsgn} for Native (I _{test} / CF _{test}):		2.00	in/hr	
I _{dsgn} for Imported Growing Medium:		2.00	in/hr	



(Presumptive Approach	Calculato	r ver. 1.1	Catchmen	·	
Proi	ect Name: Milwaukie Park - Plaza 1	с	atchment ID:	Run Time: 12/10/. P1 Date:	12/10/2009	
1 1 2 3 4 5	nstructions: . Identify which Stormwater Hierarchy Categor . Select Facility Type. . Identify facility shape of surface facility to mo and sloped planters that use the PAC Sloped . Select type of facility configuration. . Complete data entry for all highlighted cells. acility will meet Hierarchy Category:	ry the facility. pre accurately es	stimate surface volum			
Goal Summa						
Hierarchy Category	SWMM Requirement	RESULTS box 1 Pollution Reduction as a	below needs to display 10-yr (aka disposal) as a	Facility configurations allowed		
I	On-site infiltration with a surface infiltration facility.	PASS	PASS	A or B		
	ty Type = <u>Swale</u> Refer to Sloped Facility Worksheet and enter Variable Parameters	Facility C	PLANTER - BASIN Facility Swall om Area GROWING MEDIUM	age Depth 1 GM Depth Veriflow		Calculation Guide Max. Rock Stor.
	ABOVE GRADE STORAGE COMPONENT Infiltration Area =788sf .ce Capacity Volume =536.4cf		<u>BELOW GRA</u> Rock Storage Bottor Rock Storage			Bottom Area Per Swale Dims
Surface GM Des	wing Medium Depth = <u>18</u> in Freeboard Depth = <u>N/A</u> in Capacity at Depth 1 = <u>536</u> cf sign Infiltration Rate = <u>2.00</u> in/hr Infiltration Capacity = <u>0.036</u> cfs	N	Rock Storage Ca ative Design Infiltratio Infiltration Ca	n Rate = <u>2.00</u> in/		Rate Used in PAC
	Overflow ESULTS Volume Pollution PASS 0 CF 1% Surf. 10-yr FAIL 33 CF 100% Surf. ACILITY FACTS Total Facility Area Includin Sizing Ratio (Total Facility Area / Catc	Cap. Used	Cu	ing - Data Modified, I rrent data has been aza1_Export.xIs		'M



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10-yr Con-A&B

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	Presumptive Approach Calculate	Catchment Data			
		Catchment ID:	P2		
Project Name:	Milwaukie Park - Plaza 2	Date:	12/10/09		
Project Address:	-	Permit Number: 0			
	Milwaukie, OR	- Run Time: 12/10/	2000 2-91-47 044		
Designer:	SDH		2000 0.21.47 1 10		
Company:	David Evans and Associates, Inc.	.			
		-			

Drainage Catchment Information										
Catchment ID	P2									
Catchment Area										
Impervious Area	2,145	SF								
Impervious Area	0.05	ac								
Impervious Area Curve Number, CN _{imp}	98									
Time of Concentration, Tc, minutes	5	min.								
Site Soils & Infiltration Testing Data										
Infiltration Testing Procedure: Open Pi	t Falling Head									
Native Soil Field Tested Infiltration Rate (Itest):	4	in/hr								
Bottom of Facility Meets Required Separation From										
High Groundwater Per BES SWMM Section 1.4:	Yes									
Correction Factor Component										
CF _{test} (ranges from 1 to 3)	2									
Design Infiltration Rates										
I _{dsgn} for Native (I _{test} / CF _{test}):	2.00	in/hr								
I _{dsgn} for Imported Growing Medium:	2.00	în/hr								



Presumptive Approach Ca			Calculato	r ver. 1.1	Catchment ID: P2 Run Time: 12/10/2009 3:21:47 PM			
Pro	oject Name:	Milwaukie Park - Plaza 2	c	atchment ID:	P2	Date:	12/10/2009	
	 Select Fa Identify fa and slope Select typ Complete 	hich Stormwater Hierarchy Categor cility Type. cility shape of surface facility to mo d planters that use the PAC Sloped e of facility configuration. data entry for all highlighted cells.	ore accurately e			Swales		
Catchment Goal Sumn		eet Hierarchy Category:	1					
Hierarchy Category	<u> </u>	SWMM Requirement	RESULTS box Pollution Reduction as a					
1	On-site infi	Itration with a surface infiltration facility.	PASS	PASS	A or B			
Faci	ility Type = :	Swale	Facility C	configuration:	A			
	Worksl	Sloped Facility neet and enter le Parameters		F DAILING ST	ASIN/ WALE Corage Depth 1 GM Depth Coverflo	A *		Calculation Guide
		RADE STORAGE COMPONENT ion Area = 407 sf v Volume = 278.2 cf		Rock Storage Bo		6 <u>5</u> 67 sf 0 in		Max. Rock Stor. Bottom Area Per Swale Dims
	rowing Mediu Freeboa ce Capacity at	rd Depth = <u>N/A</u> in		Rock Storage	• Capacity =	0 cf		
GM D	Design Infiltrat	<u> </u>	1	Native Design Infiltration		. <u>00 </u> in/hr 019 cfs	GM Infiltration R	ate Used in PAC
	RESULTS Pollution Reduction 10-yr	Overflow Volume PASS 0 CF 0% Surf. PASS 0 CF 12% Surf.		Run PAC	-			
	FACILITY F	ACTS		<u> </u>	Current data h			.
	Siz	Total Facility Area Includin ing Ratio (Total Facility Area / Catc	-	748 SF 0.349	Plaza2_Export	LXIS 12/	10/2009 3:29:06 PM	



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Stoped Facility Worksheet



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